

Appendix 2

Approved Drainage Proposal under Planning Application No. A/YL-SK/430

-TEMPORARY DRAINAGE PROPOSAL

APPLICATION SITE OF PROPOSED TEMPORARY
PUBLIC VEHICLE PARK AND ASSOCIATED
FILLING OF LANDS FOR A PERIOD OF 3 YEARS
AT LOTS 1054S.B (PART) IN D.D. 106 AND
ADJOINING GOVERNMENT LAND, SHEK KONG,
YUEN LONG, NEW TERRITORIES

Application No.: A/YL-SK/430

Project No.: ALPL/TDM/017

Revision No.: 0

17 Sep 2025

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2 Introduction

2.1 Background

This report presents the Drainage Proposal for supporting the Proposed temporary public vehicle park and associated filling of lands for a period of 3 years at Lots 1054 S.B(Part) in D.D. 106 and adjoining government land, Shek Kong, Yuen Long, New Territories.

2.2 Objectives of the Report

This report shall be prepared to include the following:

- Identify the potential drainage impact assessment from the proposed Application Site
- Recommend and implement all necessary measures to mitigate adverse drainage impacts arising from the application site

2.3 Report Structure

The report contains the following sections:

- Section 1 on Introduction;
- Section 2 on Development Proposal;
- Section 3 on Assessment Criteria;
- Section 4 on Potential Drainage Impact; and
- Section 5 on Conclusion.

3 Development Proposal

3.1 Location of the Application Site

The application Site is located within the Kam Sheung Road, with an area of around 3221m² and ground level varying between + 11.9mPD and + 11.7mPD. The layout plan is provided in **Appendix B**.

This application site is "Agriculture" zoning, the type of application is the Temporary Use/Development in Rural Areas for a Period of 3 Years.

There is a existing catchpit vicinity of the application site, the location and site photos of the existing catchpit are provided in **Appendix C**.

4 Assessment Criteria

4.1 Design Return Periods

The drainage system in the Application site is to collect surface flows and convey to the existing catchpit

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and finally convey to downstream village drain. The recommended design return periods based on the flood levels for the various drainage systems depend on the drainage system, land use, hazard to public safety and community expectations. The recommended design return period is reproduced in Table 4-1 below:

DESCRIPTION	DESIGN RETURN PERIODS
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage including internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 4-1 Recommended Design Return Periods based on Flood Levels

As per Storm Drainage Manuel (SDM) Section 6.6.2 Urban Drainage Branch and Urban Drainage Trunk Systems “An ‘Urban Drainage Branch System’ is defined as a group or network of connecting drains collecting runoff from the urban area and conveying stormwater to a trunk drain, river or sea. For a simple definition, the largest pipe size or the equivalent diameter in case of a box culvert in a branch system will normally be less than 1.8m.

An ‘Urban Drainage Trunk System’ collects stormwater from branch drains and/or river inlets, and conveys the flow to outfalls in river or sea. Pipes with size or diameter equal to or larger than 1.8m are normally considered as trunk drains.”

As per SDM, since the proposed U-channels are sized smaller than 1.8m, the drainage system would be defined as an urban drainage branch with recommended design return period of 50 years.

The 50 years design return period will be considered to ensure adequacy of the stormwater drainage system.

4.2 Calculation Methodology for Runoff

Peak instantaneous runoff values before and after the development were calculated based on the Rational Method and with recommended physical parameters including runoff coefficient (C) and storm constants for different return periods referred to the SDM, based on the following equation:

$$Q_p = 0.278 C i A$$

where	Q_p	=	Peak Runoff, m ³ /s
	C	=	Runoff Coefficient
	i	=	Rainfall Intensity, mm/hr
	A	=	Catchment Area, km ²

The paved area of the site will account for 3221m². For conservative, the runoff coefficient of 0.9 is assumed, such that the all the run-off would be collected from the catchment area without any infiltration as the critical scenario.

Based on the storm constants for 50-year return period recommended in the SDM, the appropriate rainfall intensities (i) are calculated as detailed in Appendix D

4.3 Calculation Methodology for Pipe Capacity Checking

Due to the catchment areas are less than 1ha, U-channels are recommended to be constructed to collect the stormwater runoff of the open area within the site. The collected stormwater would be diverted to 375mm U-channel system at boundary of application site. The collected stormwater should finally be diverted to the downstream via the proposed U-channel system.

For the worst-case scenario, bad condition of concrete pipe is assumed for the Manning's roughness coefficient (coefficient value is 0.016) for calculating capacities of concrete U-channel using Manning's Equation.

Manning's Equation for calculating the channel and pipe capacities is adopted.

5 Potential Drainage Impact

5.1 Existing Site Condition

The application Site is located within the Tai Tong with an area of around 3880m² and ground level varying between + 12.1mPD and + 11.8mPD.

5.2 Changes in Drainage Characteristics

Since the ground level of application site is generally higher than the adjacent ground surface. No

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external catchment shall be considered in the calculation.

The characteristics of the sub-catchment areas are altered due to the proposed application, which are changed from unpaved site area to paved area. The change in sub-catchment is summarized in Table 5-1.

	Before	After
Grassland (m ²)	0	0
Paved Area (m ²)	3221	3221
External Catchment Area(m ²)	0	0
Total Catchment Area (m ²)	3221	3221

Table 5-1 Change in sub-catchment within the site

5.3 Potential Drainage Impact

The details of the proposed drainage works are illustrated in **Appendix C**.

To effectively convey stormwater away from the application site and minimize the potential impact to the drainage infrastructure of the village area, drainage works consists of U-channels, are proposed to convey the stormwater runoff to the terminate catchpit with sand trap (TCP), and finally convey to the existing catchpit.

The runoff within application site is collected by two 375mm U-channels along the boundary and convey to the existing terminate catchpit with sand trap (TCP)within the application site, before discharging to the existing catchpit at the South direction of the application site, and eventually discharge to the further downstream village drain.

as indicated in the **Appendix C**.

Catchment Area	Drainage System	Estimated Flow (L/min)	Capacity (L/min)	Reserve Capacity
1	375mm u-channel	7657	14000	45%
2	375mm u-channel	9736	14000	30%
1+2	450mm discharge pipe(existing)	17393	20039	14%

Table 4-2 Design calculation of the proposed drainage work

1. Rainfall increase due to climate change at the end of 21st century is considered according to stormwater drainage manual Table 28.
2. The reserve capacity is calculated by assuming that the U-channel reach its full capacity.

The design runoff arise from the proposed catchment area 1 is to be discharged into the proposed

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375mm u-channel within the runoff anticipated to be of 7657L/min, which is within the drainage capacity of the proposed 375mm u-channel of 14000L/min, the reserve capacity is 45%.

The design runoff arise from the proposed catchment area 2 is to be discharged into the proposed 375mm u-channel within the runoff anticipated to be of 9736L/min, which is within the drainage capacity of the proposed 375mm u-channel of 14000L/min, the reserve capacity is 30%.

The runoff would then be convey to the existing terminal catchpit within the application site, and discharge to downstream village drain by 450mm existing discharge pipe. The total runoff to be discharged by the discharge pipe is 17393L/min, and the capacity of discharge pipe is 20039L/min, the reserve capacity is 16%.

It is considered that the drainage discharge from the Application Site will not cause adverse impact to the entire downstream drainage system.

All u-channels & catch pits will be constructed according to the CEDD's standard drawings, please refer to the **Appendix E**.

6 Construction Stage

6.1 Temporary Drainage Arrangements

Proper measures shall be taken to maintain the existing drainage characteristics of the catchment areas and to minimize drainage impacts associated with the construction works. The principal drainage impacts which are associated with construction of the works have been identified as follows:

- (i) Erosion of ground materials;
- (ii) Sediment transportation to existing downstream drainage system; and
- (iii) Obstruction to drainage systems.

Regular inspections shall be carried out to ensure integrity of the works. These inspections shall cover works under construction as well as recently completed areas.

To ensure proper operation of the site drainage channels and desilting facilities, inspection of the perimeter drains shall be carried out on a weekly basis and the desilting facilities shall be cleaned on a daily basis.

If excavated materials are not possible to transport away the excavated material within the same day, the material should be covered by tarpaulin/impervious sheets. Stockpiles of construction materials (for examples aggregate, fill materials) of more than 50 m³ in an open area shall also be covered with

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tarpaulin or similar fabric during rainstorms.

All runoff discharged into the existing drainage system will be settled in a silt trap to ensure no sediment will be discharged into the channel. Silt traps will normally be provided along the site drainage immediately upstream of the proposed discharge point to the existing Site. The silt traps will be inspected daily and immediately after each rainstorm.

Liaison will be carried out with relevant parties regarding temporary drainage arrangements to ensure that the drainage system is functioning adequately.

7 Conclusions

7.1 Conclusions

The analyzed catchment area of 3221m² consists of the site area of the proposed Application Site only and no external catchment area had been identified.

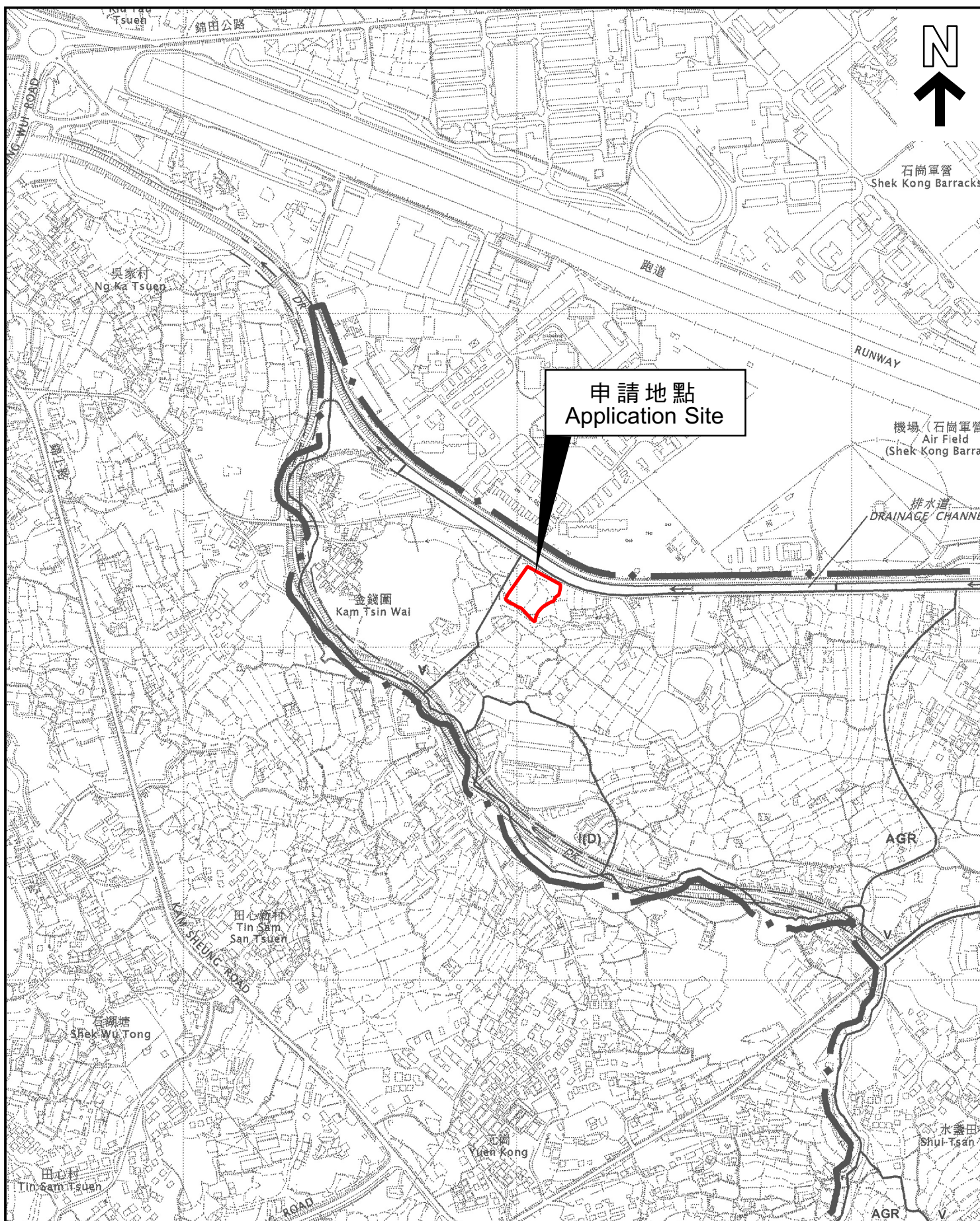
All round U-channels are proposed to convey runoff from the application site for collection. The proposed U-channels are located along the boundary of application site which is subject to change to suit the building layout.

The assessment reviews the drainage pipe have the sufficient capacity to cater for the drainage flow from the Application Site.

Mitigation measures are proposed during the construction period and to ensure that the existing drainage system within the site will not be affected during the construction stage.

APPENDIX A

SITE LAYOUT PLAN



本摘要圖於2025年7月25日擬備，
所根據的資料為於2006年10月17日
核准的分區計劃大綱圖編號S/YL-SK/9
EXTRACT PLAN PREPARED ON
25.7.2025 BASED ON OUTLINE ZONING
PLAN No. S/YL-SK/9 APPROVED ON
17.10.2006

位置圖 LOCATION PLAN

SCALE 1 : 7 500 比例尺

米 100 0 100 200 300 米
METRES METRES

申請地點界線只作識別用
APPLICATION SITE BOUNDARY
FOR IDENTIFICATION PURPOSE ONLY

參考編號
REFERENCE No.

A/YL-SK/430

APPENDIX B

LAYOUT PLAN

DEVELOPMENT PARAMETERS

APPLICATION SITE AREA	:	3,221 m ²	(ABOUT)
COVERED AREA	:	60 m ²	(ABOUT)
UNCOVERED AREA	:	3,161 m ²	(ABOUT)

PLOT RATIO	: 0.02	(ABOUT)
SITE COVERAGE	: 1.9%	(ABOUT)

NO. OF STRUCTURE	: 2
DOMESTIC GFA	: NOT APPLICABLE
NON-DOMESTIC GFA	: 60 m ² (ABOUT)
TOTAL GFA	: 60 m ² (ABOUT)

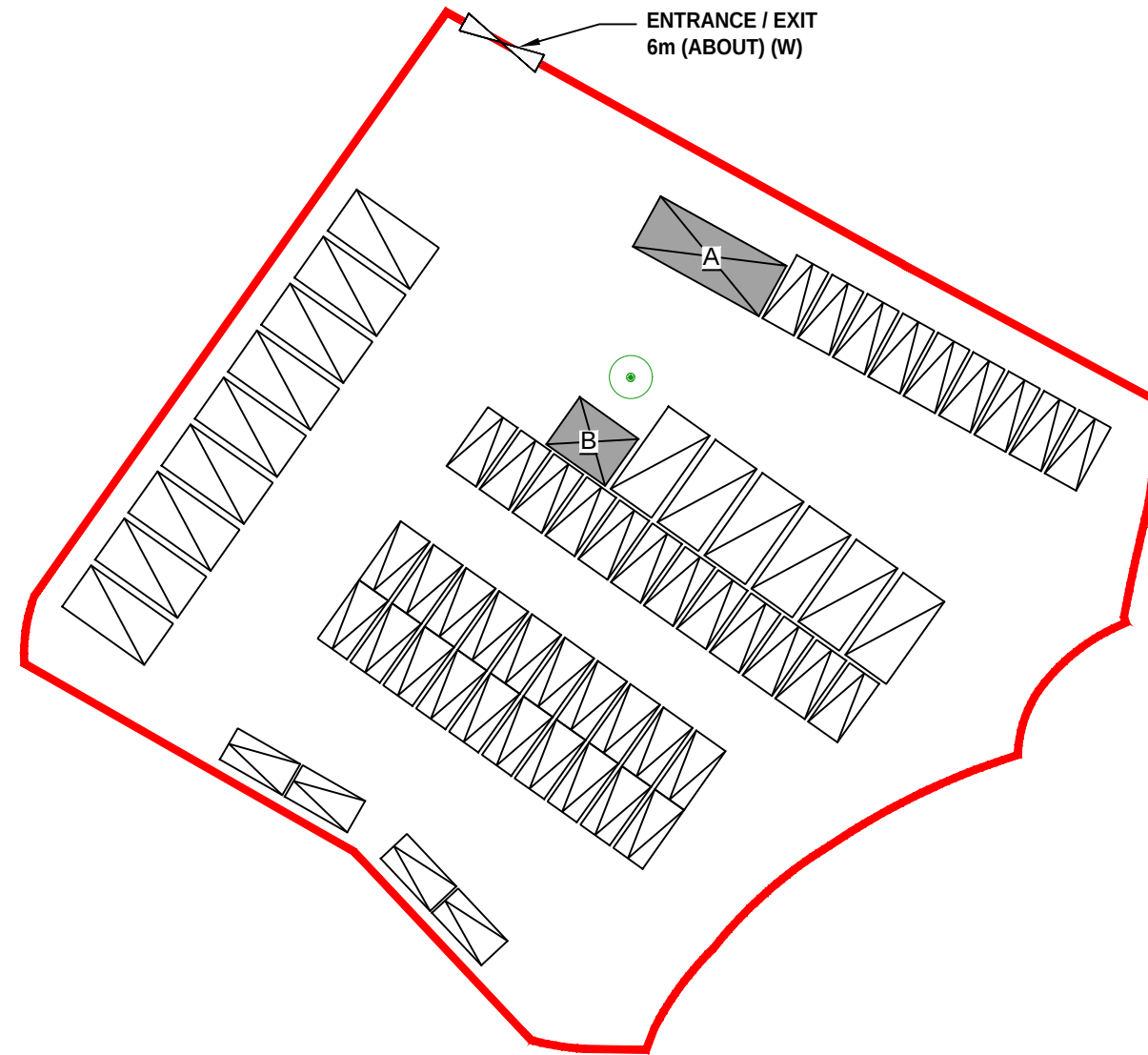
BUILDING HEIGHT : 4 m (NOT MORE THAN)
NO. OF STOREY : 1 (NOT MORE THAN)

STRUCTURE	USE	COVERED AREA	GFA	BUILDING HEIGHT
A	SHROFF AND ANCILLARY OFFICE	40 m ² (ABOUT)	40 m ² (ABOUT)	4 m (NOT MORE THAN) (1-STOREY)
B	STORE ROOM	20 m ² (ABOUT)	20 m ² (ABOUT)	4 m (NOT MORE THAN) (1-STOREY)
TOTAL		60 m ² (ABOUT)	60 m ² (ABOUT)	

PARKING PROVISIONS

NO. OF PRIVATE CAR PARKING SPACE : 45
DIMENSIONS OF PARKING SPACE : 5 m (L) X 2.5 m (W)

NO. OF LIGHT GOODS VEHICLES PARKING SPACE : 15
DIMENSIONS OF PARKING SPACE : 7 m (L) X 3.5 m (W)

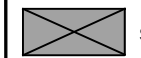


- ALL DIMENSIONS ARE IN MILLIMETER EXCEPT OTHERWISE NOTED
- DO NOT SCALE DRAWING

LEGEND



APPLICATION SITE



STRUCTURE



PARKING SPACE
(Private Car)
2.5 m (W) X 5 m (L)



PARKING SPACE
(Light Goods Vehicles)
3.5 m (W) X 7 m (L)



ENTRANCE / EXIT



EXISTING TREE

-	LAYOUT PLAN	19 JUN 2025

REVISION	
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	name	
drawn	CY	DATE 19 JUN 2025
checked	-	DATE
approved	-	DATE

contract no. -

project no.	DD106
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TITLE :
PROPOSED TEMPORARY PUBLIC VEHICLE PARK
(EXCLUDING CONTAINER VEHICLE) AND
ASSOCIATED FILLING OF LAND FOR A PERIOD OF 3
YEARS

SITE LOCATION :
LOTS 1054 S.B AND 1056 S.B (PART) IN D.D. 106,
SHEK KONG, YUEN LONG, NEW TERRITORIES

drawing title.

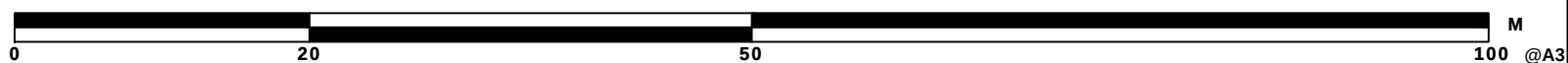
LAYOUT PLAN

drawing no.

PLAN 1

rev.	scale
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-	N.T.S.
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APPENDIX C

PROPOSED DRAINAGE PLAN

LAND STATUS

APPLICATION SITE AREA : 3,221 m² (ABOUT)
AREA OF PRIVATE LAND : 3,221 m² (ABOUT)
AREA OF GOVERNMENT LAND : N/A



Existing Catchpit

Proposed Catchpit with Sand Trap
(Ref CEDD Drawing No. C2406/1)
Discharge to existing catchpit
CP2
C.L. +11.7mPD
I.L. +11.1mPD

Proposed Catchpit with Sand Trap
(Ref CEDD Drawing No. C2406/1)
Discharge to existing catchpit
TCP
C.L. +11.8mPD
I.L. +10.8mPD

Proposed 375mm Surface
U-channel (Refer CEDD Standard
drawing No. C2409J)
Upstream I.L. +11.5mPD
Downstream I.L. +11.1mPD

Catchment Area CA1
Approx. 1418 m²

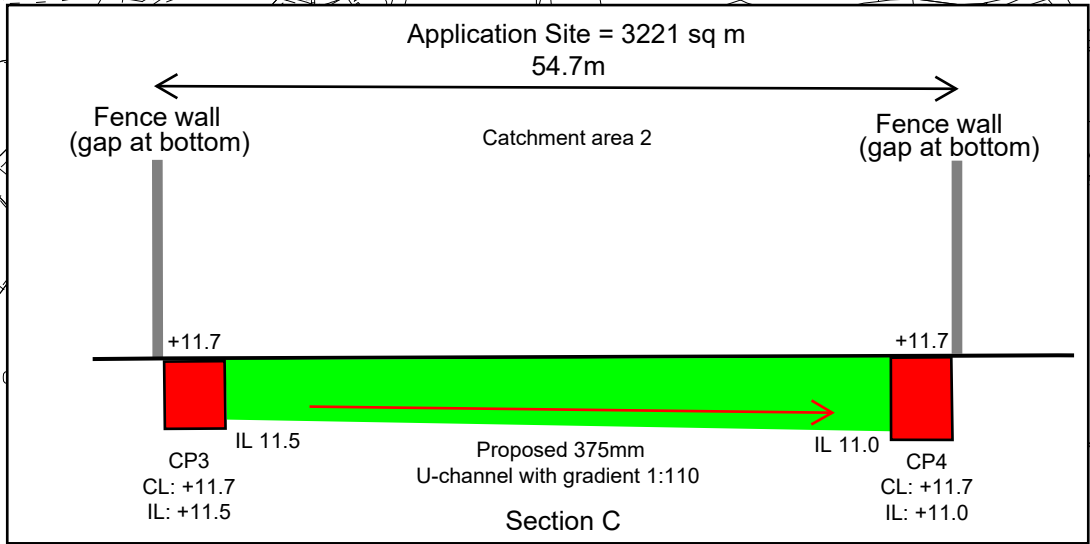
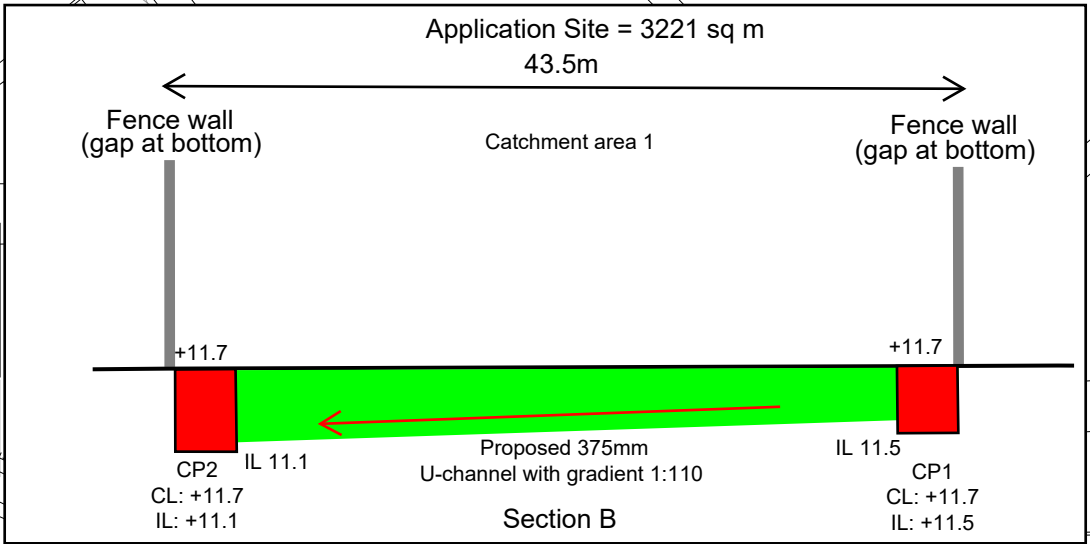
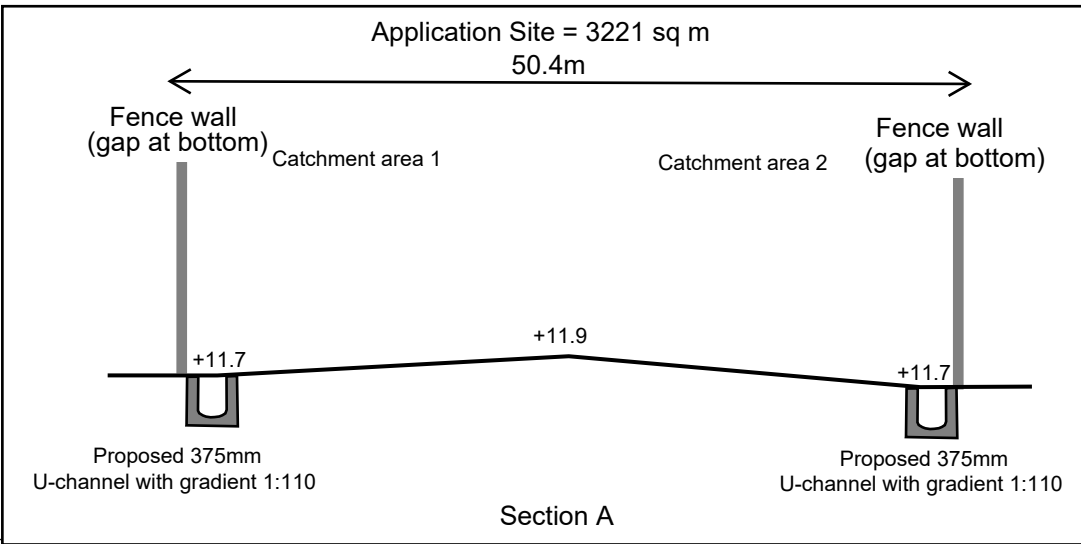
Proposed Catchpit with Sand Trap
(Ref CEDD Drawing No. C2406/1)
Discharge to existing catchpit
CP1
C.L. +11.7mPD
I.L. +11.5mPD

Proposed Catchpit with Sand Trap
(Ref CEDD Drawing No. C2406/1)
Discharge to existing catchpit
CP3
C.L. +11.7mPD
I.L. +11.5mPD

Proposed Catchpit with Sand Trap
(Ref CEDD Drawing No. C2406/1)
Discharge to existing catchpit
CP2
C.L. +11.7mPD
I.L. +11.0mPD

Proposed 375mm Surface
U-channel (Refer CEDD Standard
drawing No. C2409J)
Upstream I.L. +11.5mPD
Downstream I.L. +11.0mPD

Catchment Area CA2
Approx. 1803 m²



ALL DIMENSIONS ARE IN MILLIMETER EXCEPT OTHERWISE NOTED
DO NOT SCALE DRAWING

LEGEND

- APPLICATION SITE
- PRIVATE LAND
- Proposed U-channel
- Proposed Carchpit
- Existing Carchpit

LAYOUT PLAN 19 JUN 2025

REVISION		
drawn	CY	DATE 19 JUN 2025
checked	-	DATE
approved	-	DATE
contract no. -		
project no. DD106		
TITLE : PROPOSED TEMPORARY PUBLIC VEHICLE PARK (EXCLUDING CONTAINER VEHICLE) AND ASSOCIATED FILLING OF LAND FOR A PERIOD OF 3 YEARS		
SITE LOCATION : LOTS 1054 S.B AND 1056 S.B (PART) IN D.D. 106, SHEK KONG, YUEN LONG, NEW TERRITORIES		
drawing title. LAND STATUS PLAN		
drawing no. PLAN 3	rev. -	scale N.T.S.

APPENDIX D

DESIGN CALCULATION OF THE PROPOSED DRAINAGE SYSTEM

PROJECT: APPLICATION SITE OF PROPOSED TEMPORARY TEMPORARY PUBLIC VEHICLE PARK AND ASSOCIATED FILLING OF LANDS FOR A PERIOD OF 3 YEARS AT LOTs 1054S.B (PART) IN D.D. 106 AND ADJOINING GOVERNMENT LAND, SHEK KONG, YUEN LONG, NEW TERRITORIES **Revision :** 0 **Date :** 13-Sep-25

JOB NO : ALPL/TDM/017

TITLE : Temporary Drainage Design Calculation **Prepare By:** JW

DETERMINE THE CATCHMENT OF AREA

CA1 = 1418 (m²)
CA2 = 1803 (m²)

DETERMINE THE INLET TIME

Section	A = Catchment of Area (m ²)	Level of remote point (mPD)	Level of inlet point (mPD)	L (m)	H (m/100m)	t _e = Time of Natural flow (min)
1	CA1	1418	11.9	24	0.83	1.74
2	CA2	1803	11.9	26.4	0.76	1.91

Ref.: Brandsby Williams Equation

Note: $t_e = \frac{0.14465 L}{H^{0.2} A^{0.1}}$
H = average slope (m per 100m), measured along the line of natural flow, from the summit of the catchment to the point under consideration
L = distance (on plan) measured on the line of natural flow between the summit and the point under consideration (m).

DETERMINE THE SIZE OF STEPPED / U-CHANNEL

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
Section No.	Ldr (m)	Lh (m)	L (m)	Gradient (1 in)	Site area (m ²)	others area (m ²)	Total area (m ²)	A (m ²)	channel size	Capacity (l/min)	Assumed Flow	t _f (min)	t _e (min)	t _c (min)	I (mm/hr)	Runoff (l/min)	Remark
1	0.40	43.50	43.50	108.75	1418.00	0.00	1418.00	1418.00	375	14000	2.10	0.35	1.74	2.09	360.00	7657.2	O.K.
2	0.50	54.70	54.70	109.40	1803.00	0.00	1803.00	1803.00	375	14000	2.10	0.43	1.91	2.34	360.00	9736.2	O.K.

Note: Ldr = Different level between U-channel section
Gradient = 1 in (Lh/Ldr)
Total area = Site area + others area
A=Cumulative area = Total area + others section area
Capacity = Refer to the extracted Figure 8.7 - Chart for the Rapid Design of Channels (Geotechnical Manual for Slopes P.253)
Assumed flow velocity = Assumed velocity of runoff
Actual Flow velocity = Refer to the extracted Figure 8.7 - Chart for the Rapid Design of Channels (Geotechnical Manual for Slopes P.253)
t_f = Flow time = L / assumed flow velocity
t_e = inlet time (time taken for flow from the remotest point to reach the most upstream point of the urban drainage system)
t_c = Time of concentration = t_f + t_e (Min. t_c = 1 min. for conservative design)
I = Intensity Refer to the extracted Figure 8.2 - Curves Showing Duration and Intensity of Rainfall in H.K. for Various Return Periods (Geotechnical Manual for Slopes P.248)
K = Runoff coefficient = 0.9 refer to Character of Surface (refer DSD(2013))
Design Return Period = 50 years
Runoff = K.I.A./3600 (l/s) = K.I.A./60 (l/min)
UC U-channel
SC Stepped channel

For section no. 1.00 , Actual Flow Velocity = 1.60 m/s < 4m/s, O.K.
For section no. 2.00 , Actual Flow Velocity = 2.35 m/s < 4m/s, O.K.

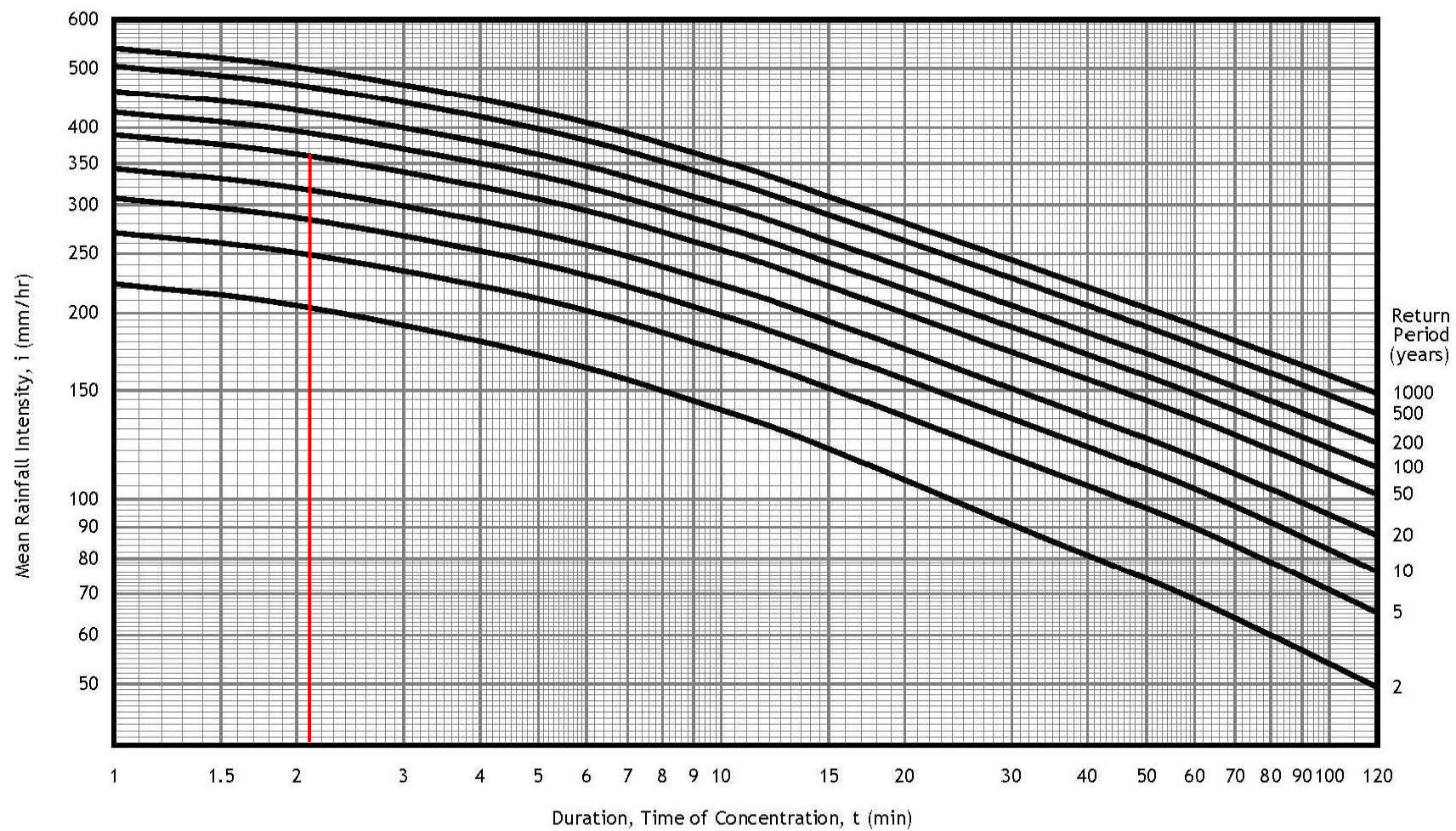
17393.4

[Use new 375 U, so it is O.K.]
[Use new 375 U, so it is O.K.]

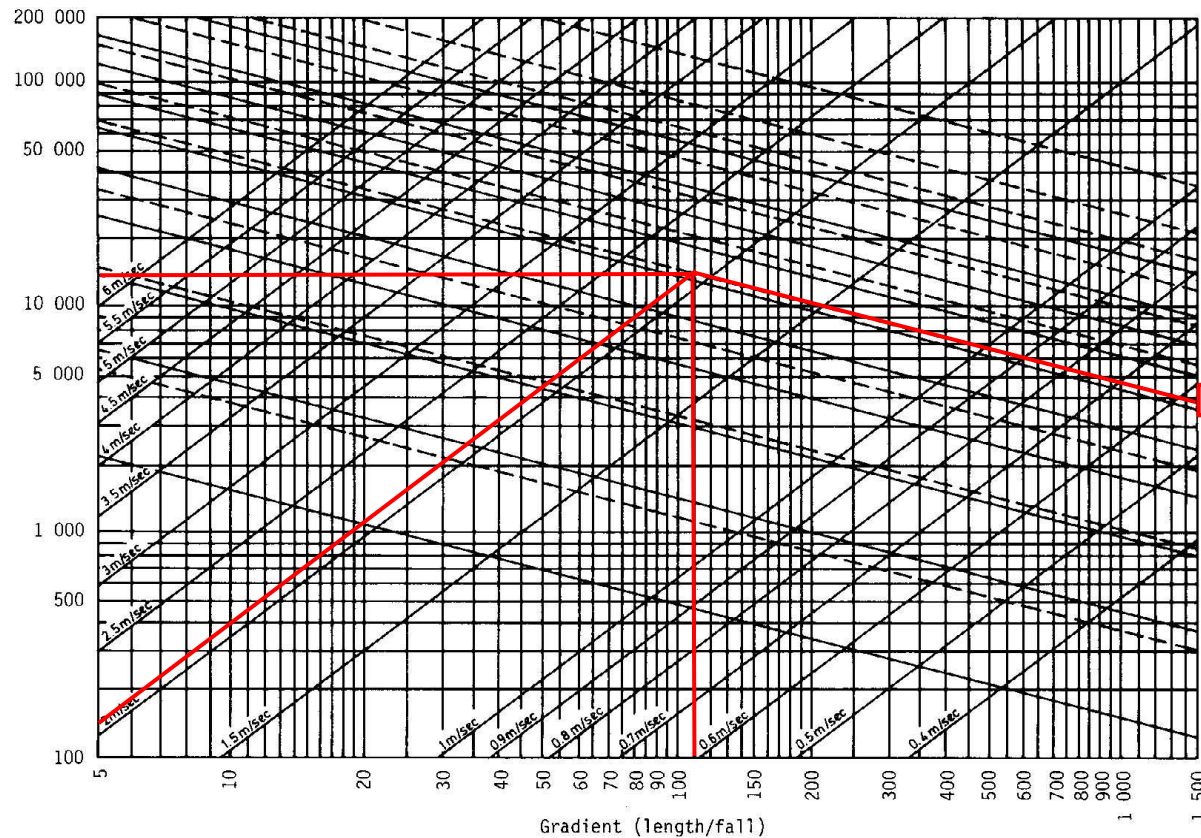
DETERMINE THE SIZE OF DISCHARGE PIPE

1	2	3	4	5	6	7	8	
Section No.	Dia. of Discharge Pipe (mm)	Sectional Area of Discharge Pipe (m ²)	v (m/s)	No. of Discharge Pipe Provided	Capacity (l/min)	Runoff (l/min)	Remark	
Existing Drainage pipe	450.00	0.16	2.10	1	20039	17393	O.K.	CAPACITY > RUNOFF [Use new 450 , so it is O.K.]

Note: v = Velocity of flow in downpipe
A = Catchment Area
K = Runoff coefficient = 0.9 refer to Character of Surface (refer DSD(2013))



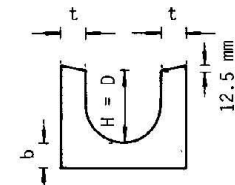
Capacities of Channels (litres per minute)



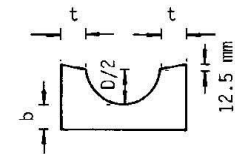
Size of channel (mm) shown by dotted solid line line

900U
750U
675U
600U
525U
450U
375U
300U
225U
150U

Channel dimensions



U-shaped channel (U)



Half-round channel (HR)

DESIGN METHOD USING CHART

(a) Normal channel Solution

1. Runoff
2. Gradient
3. Channel size
4. Velocity

Example :

1. Enter Runoff = 4 000 litre/min.
2. Enter Gradient = 1 in 40
3. Read channel required = 225 U or 300HR
4. Read velocity = 2.2 m/sec. (< 4 m/sec. ∴ OK)

(b) Stepped channel Solution

2. Runoff
3. Channel size
4. Gradient
1. Velocity

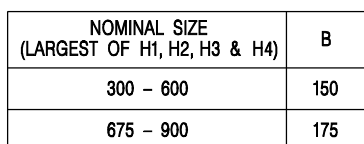
Example :

1. Enter Velocity = 5 m/sec.
2. Enter Runoff = 20 000 litre/min.
3. Read required channel size = 300U
4. Read required gradient = 1 in 14

APPENDIX E

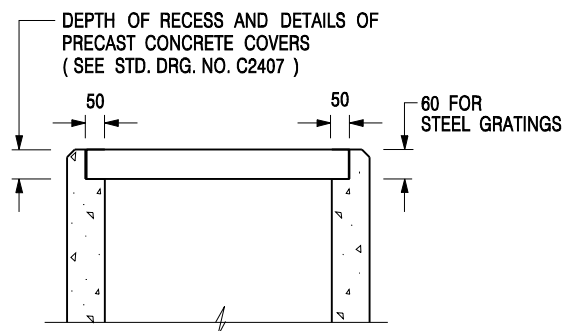
TYPICAL STANDARD DRAWINGS OF U-CHANNEL AND CATCHPIT

(EXTRACTED FROM CEDD, FOR REFERENCE ONLY)



- CATCHPIT WITH TRAP
(SHEET 1 OF 2)

-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE
 CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT			
SCALE 1 : 20		DRAWING NO.	
DATE JAN 1991		C2406 /1	



ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE SHALL BE GRADE 20 /20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
5. CONCRETE TO BE COLOURED AS SPECIFIED.
6. UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

A	MINOR AMENDMENT.	Original Signed	04.2016
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE

**CATCHPIT WITH TRAP
(SHEET 2 OF 2)**



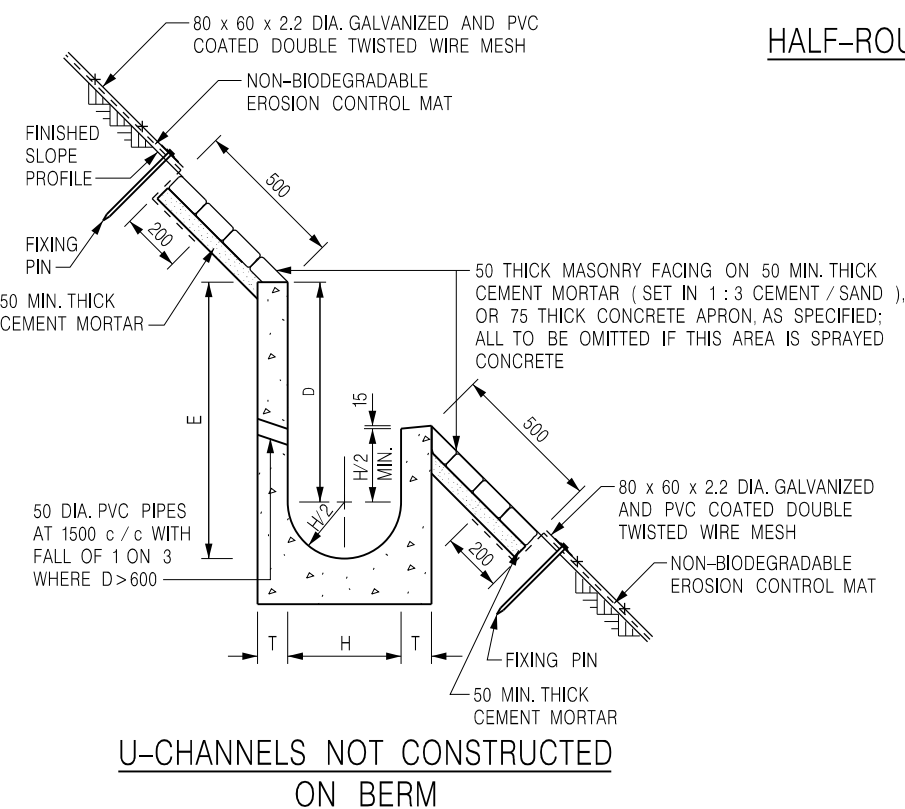
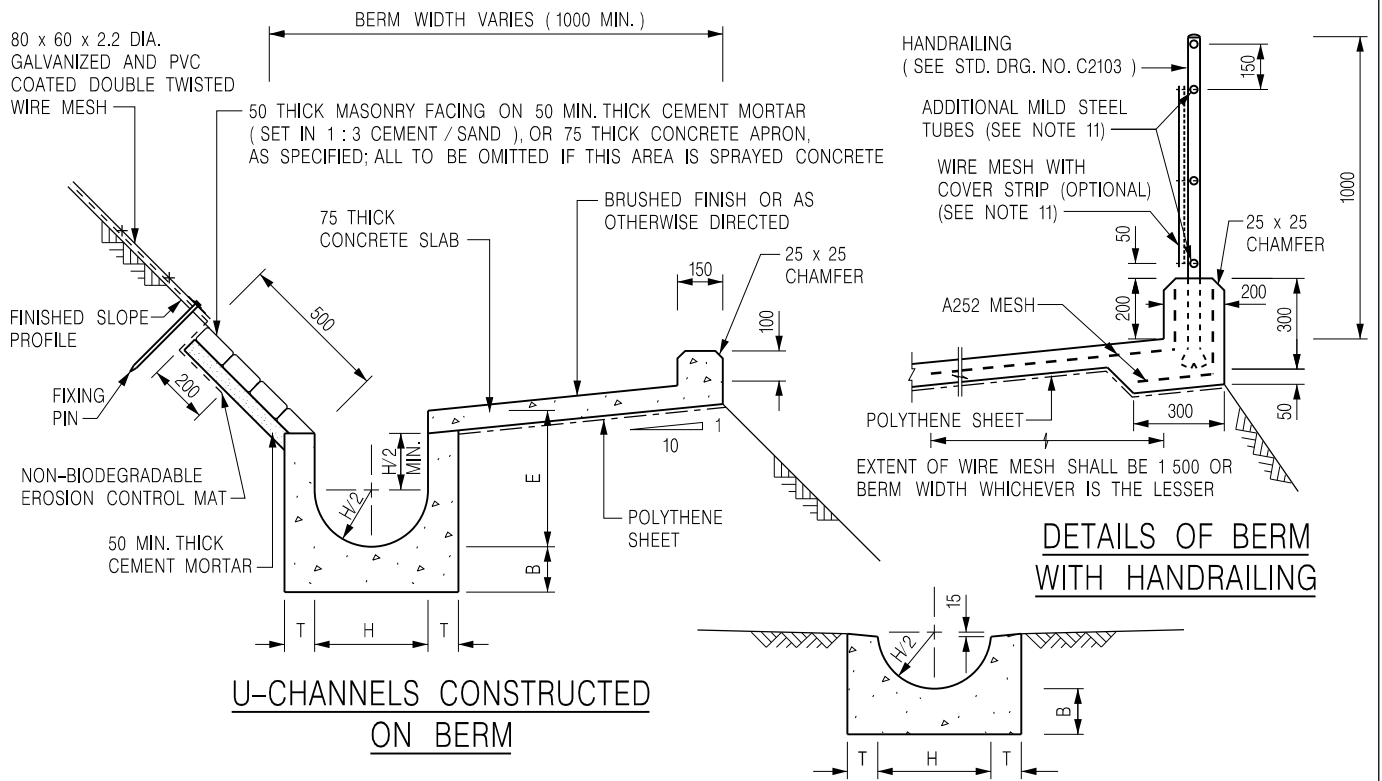
**CIVIL ENGINEERING AND
DEVELOPMENT DEPARTMENT**

SCALE 1 : 20

DATE JAN 1991

DRAWING NO.

C2406 /2A



NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE TO BE GRADE 20 / 20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
4. SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
7. BIODEGRADABLE EROSION CONTROL MAT IF REQUIRED, SEE STD. DRG. NO. C2511/E.
8. CONCRETE TO BE COLOURED AS SPECIFIED.
9. CONCRETE U-CHANNEL CAN BE CAST IN-SITU OR PRECAST CONCRETE SUBJECT TO THE ENGINEER'S AGREEMENT ON THE DETAILS.
10. DETAILS OF EROSION CONTROL MAT AND WESH MESH ON BERM. (SEE STD. DRG. NO. C2511/E)
11. THE WIRE MESH ON HANDRAILING IS OPTIONAL. THE COVER STRIP AND ADDITIONAL MILD STEEL TUBES ARE NEEDED ONLY IF WIRE MESH IS PROVIDED. (SEE STD. DRG. NO. C2103)

NOMINAL SIZE H	T	B	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100 WHEN E>650
375 - 600	100	150	
675 - 900	125	175	A252 MESH PLACED CENTRALLY

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A - WITH MASONRY APRON)

J	DETAILS OF HANDRAILING AMENDED.	Original Signed	08.2024
I	MINOR AMENDMENT.	Original Signed	07.2018
H	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
G	MINOR AMENDMENT.	Original Signed	01.2004
F	GENERAL REVISION.	Original Signed	12.2002
REF.	REVISION	SIGNATURE	DATE



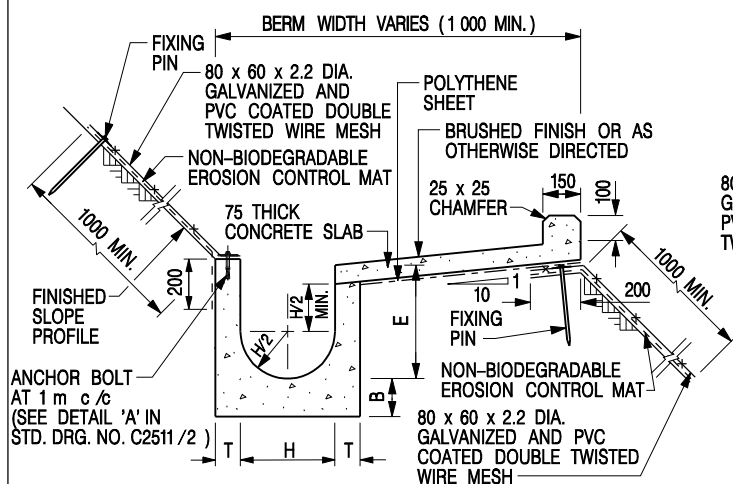
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SCALE 1 : 25

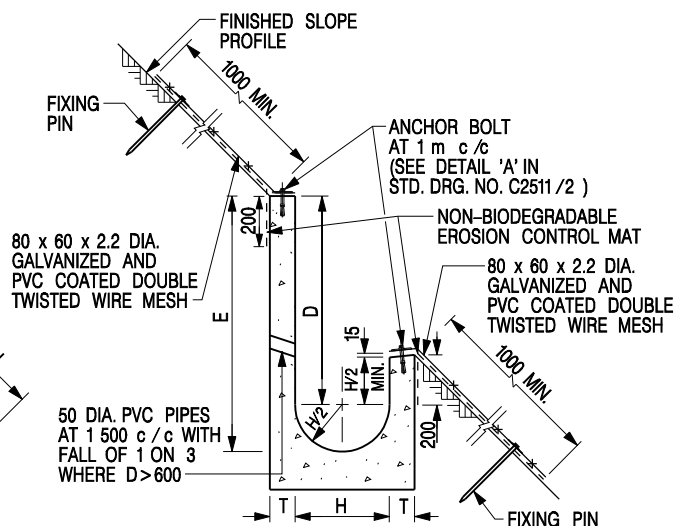
DATE JAN 1991

DRAWING NO.

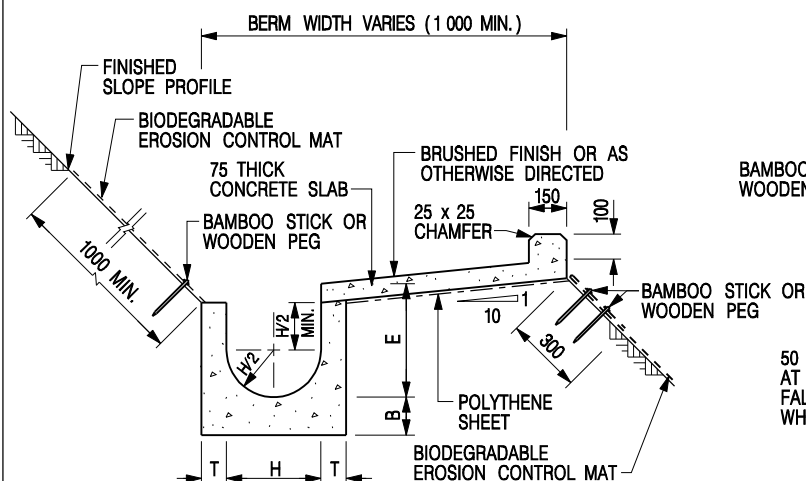
C2409J



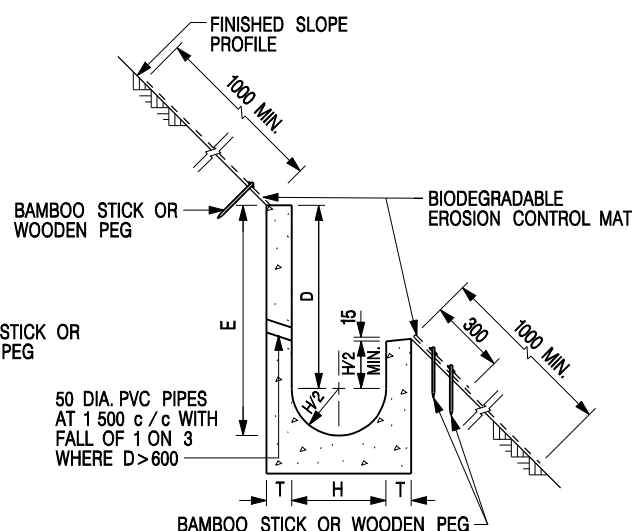
**U-CHANNELS CONSTRUCTED ON BERM
WITH NON-BIODEGRADABLE
EROSION CONTROL MAT**



**U-CHANNELS NOT CONSTRUCTED ON BERM
WITH NON-BIODEGRADABLE
EROSION CONTROL MAT**



**U-CHANNELS CONSTRUCTED ON BERM
WITH BIODEGRADABLE
EROSION CONTROL MAT**



**U-CHANNELS NOT CONSTRUCTED ON BERM
WITH BIODEGRADABLE
EROSION CONTROL MAT**

NOTES:

- ALL DIMENSIONS ARE IN MILLIMETRES.
- ALL CONCRETE TO BE GRADE 20 /20.
- CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

NOMINAL SIZE H	T	B	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100 WHEN E > 650
375 - 600	100	150	
675 - 900	125	175	A252 MESH PLACED CENTRALLY

I	MINOR AMENDMENT.	Original Signed	07.2018
H	FIXING DETAILS OF BIODEGRADABLE EROSION CONTROL MAT ADDED.	Original Signed	12.2017
G	DIMENSION TABLE AMENDED.	Original Signed	01.2005
F	MINOR AMENDMENT.	Original Signed	01.2004
E	GENERAL REVISION.	Original Signed	12.2002
D	MINOR AMENDMENT.	Original Signed	08.2001
C	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
B	MINOR AMENDMENT.	Original Signed	3.94
A	MINOR AMENDMENT.	Original Signed	10.92
REF.	REVISION	SIGNATURE	DATE

**DETAILS OF HALF-ROUND AND
U-CHANNELS (TYPE B - WITH
EROSION CONTROL MAT APRON)**



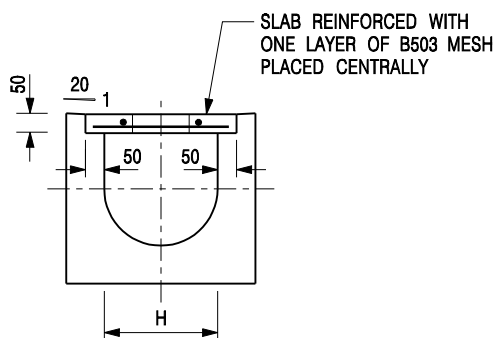
**CIVIL ENGINEERING AND
DEVELOPMENT DEPARTMENT**

SCALE DIAGRAMMATIC

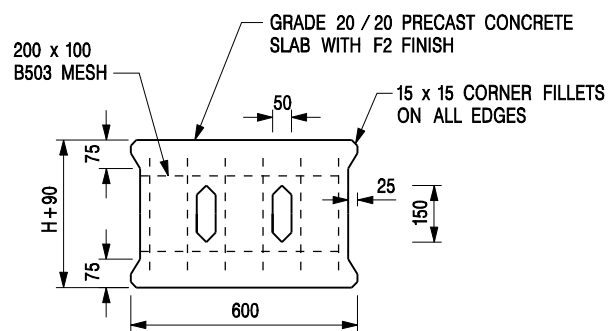
DRAWING NO.

DATE JAN 1991

C24101



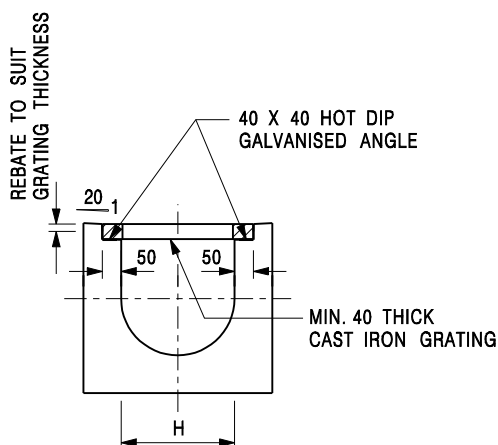
TYPICAL SECTION



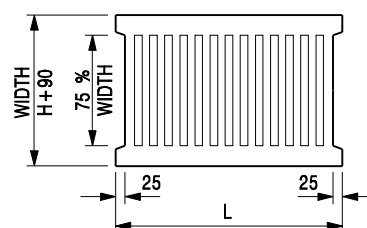
PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)



TYPICAL SECTION



L = 600mm FOR H ≤ 375mm
L = 400mm FOR H > 375mm

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- ALL DIMENSIONS ARE IN MILLIMETRES.
- H=NOMINAL CHANNEL SIZE.
- ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

E	NOTES 3 & 4 AMENDED.	Original Signed	12.2014
D	NOTE 4 ADDED.	Original Signed	06.2008
C	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
B	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
A	CAST IRON GRATING AMENDED.	Original Signed	12.2002
REF.	REVISION	SIGNATURE	DATE

**COVER SLAB AND CAST IRON
GRATING FOR CHANNELS**



**CIVIL ENGINEERING AND
DEVELOPMENT DEPARTMENT**

SCALE 1 : 20

DATE JAN 1991

DRAWING NO.
C2412E